



**US Army Corps  
of Engineers**

Hydrologic Engineering Center

---

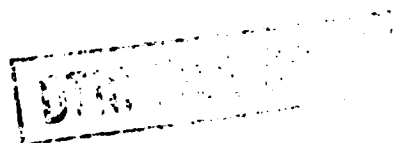
# Annual Extreme Lake Elevations by Total Probability Theorem

**Technical Paper No. 134**

May 1990

DTIC  
S ELECTE D  
MAY 13 1991  
E

Approved for Public Release. Distribution is Unlimited.



91 5 10 050

Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution within the Corps of Engineers

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

# ANNUAL EXTREME LAKE ELEVATIONS BY TOTAL PROBABILITY THEOREM

*Harold E. Kubik, P.E.\**

**ABSTRACT:** Annual extreme water levels on the Great Lakes, whether maximums or minimums, have a high serial dependence. Therefore, application of traditional frequency analysis techniques must be interpreted in a different manner and more sophisticated statistical techniques must be applied to account for this dependence.

The terms "Percent Chance Exceedance" and "Return Period" are applied to the expectation values of annual extreme events that are random in nature and have an equal likelihood of occurring in any given year. Annual extreme lake elevations on the Great Lakes are not random from one year to the next; therefore, the usual terms to define the expectation should not be used to describe the events. An acceptable term is "Percent of Years Exceeded." This is comparable to the label "Percent to Time Exceeded" that is applied to flow- or elevation-duration curves.

Decomposition of the annual extremes into two parts, one containing the highly dependent part and the other containing the random part, is one method of dealing with the dependence in the lake elevations. Appropriate statistical analyses can be applied to the separate parts and then the individual results combined to obtain the final frequency relation. This study develops mean monthly lake elevation duration curves to represent the dependent part and wind setup frequency curves for the random part. These parts are then combined by application of the total probability theorem.

Seasonality of the occurrence of both parts was found to be very important. Therefore, the complete analysis was done for the six-month fall-winter period and the six-month spring-summer period. The two curves were combined by the union of probabilities.

This technique does not gain any information over a smooth curve drawn through the observed events when applied to long-record gauges like Cleveland and Buffalo harbor. This technique is most useful in application to short-record stations. The long record of monthly lake elevations for a particular lake provides the information for the highly dependent part. The wind setup information for a short-record gauge may be correlated with a nearby long-record gauge to be made more indicative of a longer record.

Application of this method to the Buffalo harbor and Cleveland gauges resulted in computed "1% of Years Exceeded" elevations of 579.79 feet (176.72 meters) and 574.72 feet (175.17 meters) (IGLD 1955), respectively.

## Introduction

The Great Lakes are an important natural resource that have attracted a variety of human activities — waterborne commerce, water supply, hydroelectric power, recreation, and habitation — to mention some of the more important ones. The wise management of the lakes and the land adjacent to these bodies of water requires some anticipation of the likely lake levels. The establishment of non-building zones, for instance, relies on an estimate of the likely maximum water levels. Planners and designers

\*Research Hydraulic Engineer, U.S. Army Corps of Engineers, Hydrologic Engineering Center, Davis, CA  
Reprinted from: Proceedings of the Great Lakes Water Level Forecasting and Statistics Symposium,  
May 17 & 18, 1990, Windsor, Ontario, Canada. Great Lakes Commission, Ann Arbor, Michigan.

involved in the location of boat harbors and depth of navigation channels need information on the expected minimum water levels. The computation of these likely levels is complicated by the long-term fluctuations of the Great Lakes' water levels.

The normal procedure of establishing zones that are subject to flooding, especially in riverine conditions, is to compute a frequency curve based on the available flood data. One of the requirements for a frequency analysis is that the events are random, independent events. The Great Lakes' water level data *do not* meet this requirement. The annual extreme values are highly correlated from year-to-year because of the strong dependence on the mean level during the year. Therefore, normal frequency analysis procedures can not be applied to these data. It is possible to use statistical analysis techniques to analyze the extremes by separating each event into two components: one the long-time scale, highly dependent fluctuation represented by mean lake elevations; and the second the short-time scale, very independent fluctuations generally caused by wind stress on the lake. These components, after individual analysis, can be recombined to provide an indication of the percent of annual instantaneous maximum events that will exceed a given elevation. Application of these techniques to the annual minimums would provide the percent of annual events that do not exceed (nonexceedance) a given elevation.

### Data Available for Analysis

Very long records, by usual hydrologic standards in the U.S., of mean monthly water levels on Lake Erie have been observed at the Cleveland and Buffalo harbor gauges. The Cleveland record is continuous since January 1860 (129 years through 1988). And, although some mean monthly values were recorded for the 1860-1869 period, the continuous record at Buffalo harbor began in March 1887 (nearly 102 years through 1988). A continuous record of annual instantaneous extremes are available for the period 1900-1988 at Buffalo harbor and for period 1904-1988 at Cleveland. Figure 1 is a plot of mean annual lake elevations at Cleveland. One could conclude from this plot that the 129 years of information

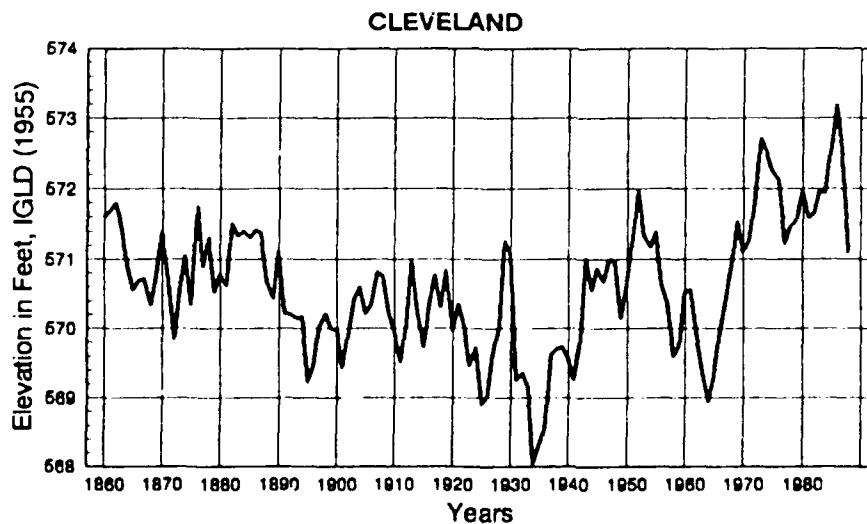


Figure 1. Mean annual elevations on Lake Erie, Cleveland gauge.

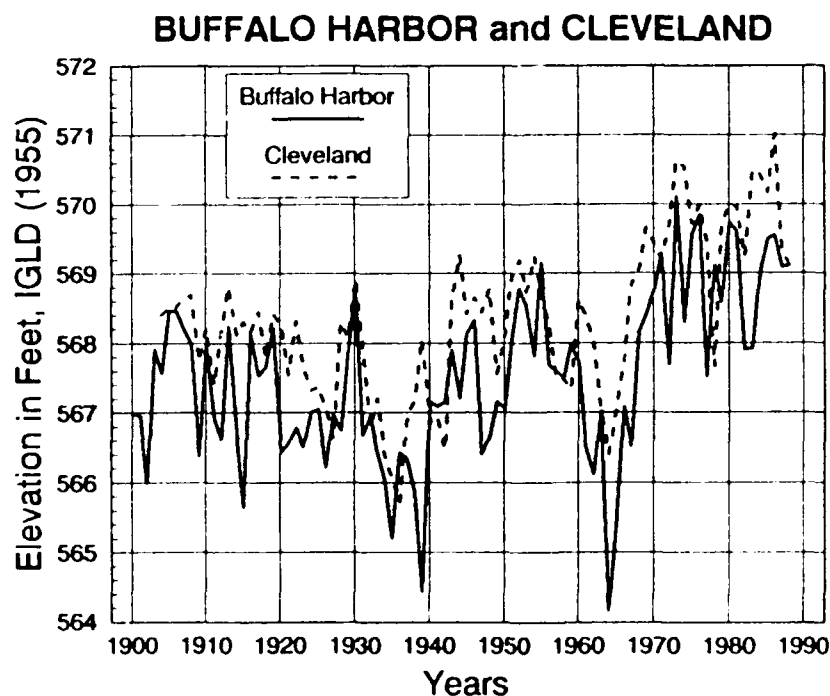
is really a very short period. The water levels in the 1860's began fairly high and gradually moved downward until the dramatic decrease in the early 1930s to a low in 1934. After this lowest annual level, the levels generally increased to the high experienced in 1986. Fitting the mean annual elevations with a smooth curve makes it appear that only one-half of a cycle has been observed. The high persistence has effectively reduced our knowledge of how often to expect extreme high or low water levels.

## Annual Persistence

Computation of the serial correlation coefficient for the annual extremes, a measure of how well one year is related to the next year, provides a quantitative evaluation of persistence. The lag 1 correlations for the annual maximum events are 0.752 and 0.406 for Cleveland and Buffalo harbor, respectively. The strength of this persistence becomes more clear when it is noted that lags 1 through 4 (this year is related to 4 years previous) are found to be significant.

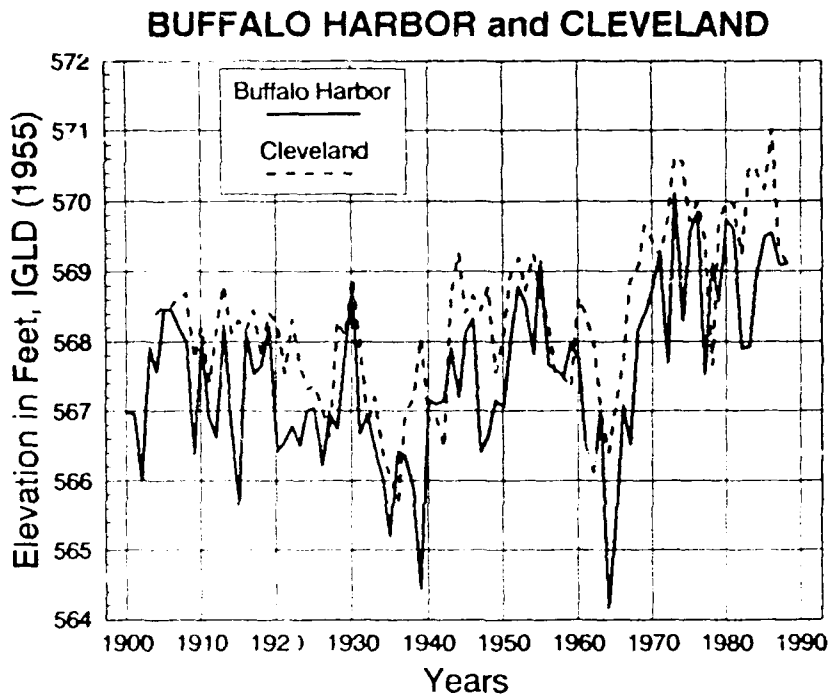
Comparison of a time series plot of the annual instantaneous extremes, Figures 2 and 3, with the mean annual values illustrate that the extremes have the same pattern as the mean annual values.

As the general lake levels are a large component of the annual extreme, then removal of this component could result in values that *do* meet the frequency requirement of being random and independent. This separation was accomplished by noting the month of the extreme, and subtracting the mean monthly water level at the gauge from the instantaneous extreme. This provided a change in elevation value that is termed "wind setup." (Note, wind setup is negative for the annual instantaneous minimums.) Serial correlation computations indicate that the wind setup values are random events; therefore, frequency analysis techniques can be applied to these data. This provides one component of the annual extreme values.



Accession For	
NTIS	GRA&I <input checked="" type="checkbox"/>
DTIC	TAB <input type="checkbox"/>
Unannounced <input type="checkbox"/>	
Justification	
By	
Distribution/	
Availability Codes	
Dist	Avail and/or Special
A-1	

Figure 2. Annual instantaneous maximums at Buffalo harbor and Cleveland.



**Figure 3. Annual instantaneous minimums at Buffalo harbor and Cleveland.**

A second component is the long-term lake fluctuations. This component is represented by a mean monthly elevation duration curve. These values are highly correlated, so the frequency label would be "Percent of Time Exceeded" to imply that they are not independent events

### Seasonality of Extremes

It became apparent as this study progressed that seasonality was important in the analysis of the extreme events. The Buffalo harbor and Cleveland maximum levels occur at entirely different times of the year. The Buffalo harbor maximums occur in the fall-winter months, indicating a response to the winter storms because the monthly lake levels are usually lower during the winter months. At Cleveland, the maximums occur in the spring-summer months indicating that the seasonal high mean lake levels are the larger determining factor. This is illustrated in Figure 4 for the maximum and minimum values at Buffalo harbor and in Figure 5 for Cleveland. For this study, the data were divided into two 6-month seasons. The fall-winter season included the months of October, November, December, January, February, and March. The spring-summer season included the months of April, May, June, July, August and September

The minimum levels are more influenced by the mean monthly lake levels, although the effect of wind related minimums can be noted at the Buffalo harbor gauge for March and April (February has the lowest average monthly elevation at both gauges).

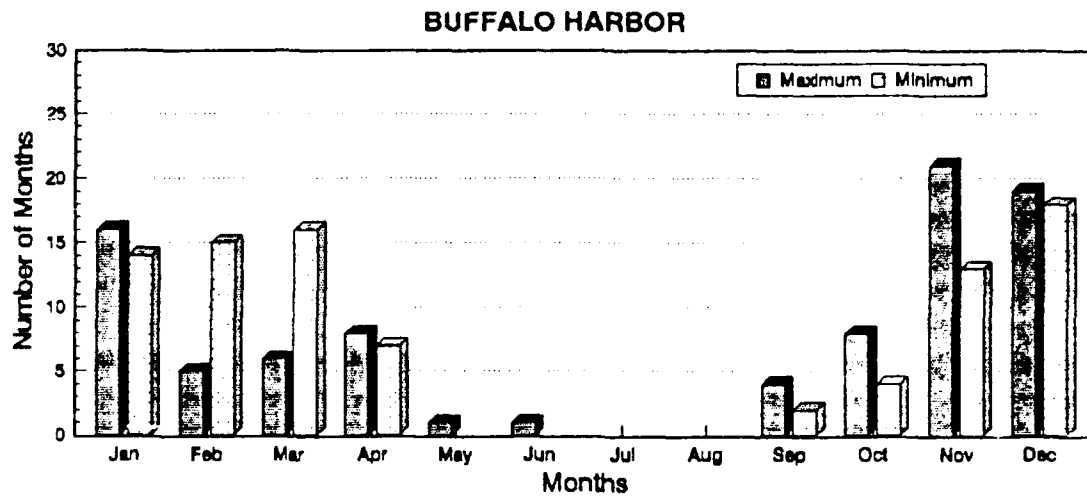


Figure 4. Months of annual maximums and minimums, Buffalo harbor.

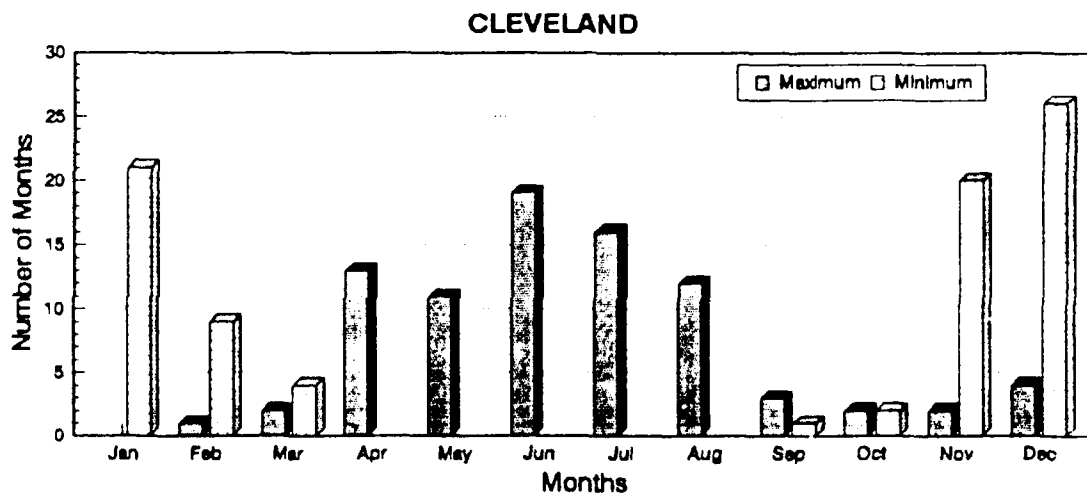


Figure 5. Months of annual maximums and minimums, Cleveland.

## Total Probability Method

Now that the annual extremes have been decomposed into two components for each of the seasons, some method must be applied to put the data back together again. This can be done by applying the total probability theorem. The total probability theorem, as presented in most statistics texts (Benjamin and Cornell 1970) is:

$$P[A] = \sum_{i=1}^n P[A | B_i] P[B_i]$$

where:

$P[A | B]$  is the conditional probability of the event  $A$  given that event  $B$  has occurred, and

$B$  is a set of mutually exclusive, collectively exhaustive events of size  $n$ .

The conditional probability relations are derived by selecting a given lake elevation and then adding this value to the wind setup frequency curve. This gives a single conditional frequency curve that has a certain probability of occurring. Many of these conditional frequency curves can be computed to completely define the range of water level occurrences. Figure 6 shows seven such conditional frequency curves. Each curve is labeled with the mean monthly lake elevation used to derive the curve and the percent of time that this elevation is exceeded. The horizontal axis (Percent of Years Exceeded) is the  $P[A | B]$  portion of the total probability equation. The  $P[B]$  portion of the equation is the amount of probability (percent of time) represented by each curve. This can, simplistically, be the probability computed by adding one-half of the differences between the two adjacent curves. For example, the probability associated with the curve based on a monthly elevation of 571.06 (exceeded 50% of the time) would be  $[(70\% - 50\%)/2 + (50\% - 30\%)/2] / 100 = 0.20$  units of probability. Doing this for all the curves will yield a set of values that add up to 1.0. In other words, all the possible mean monthly elevations have been considered by discrete increments of probability.

The total probability equation is applied at each desired elevation to compute an expectation of that elevation being exceeded. To derive a frequency relation, several elevations would be selected covering the expected range of values. Figure 7 illustrates in a graphical way what the equation is doing. An elevation of 574.0 was selected, then the Percent of Years Exceeded for each curve is noted and plotted on Figure 7 against the Percent of Time (converted to probability by dividing by 100). After all of the intercepts have been plotted, a smooth curve is drawn through the points. (Note that not all of the curves used to develop Figure 7 are shown on Figure 6.) For an elevation of 574.0, the expected Percent of Years Exceeded of 4.37% is the probability weighted average, or the area under this curve.

This computational procedure is often called coincident frequency analysis in Corps of Engineers publications. As these computations are laborious, a computer program has been written (HEC 1989) that accepts as input the mean monthly elevation-duration relation and the wind setup frequency relation. The program then generates the requisite conditional curves and evaluates the total probability theorem for several elevations to provide an elevation expectation relation.



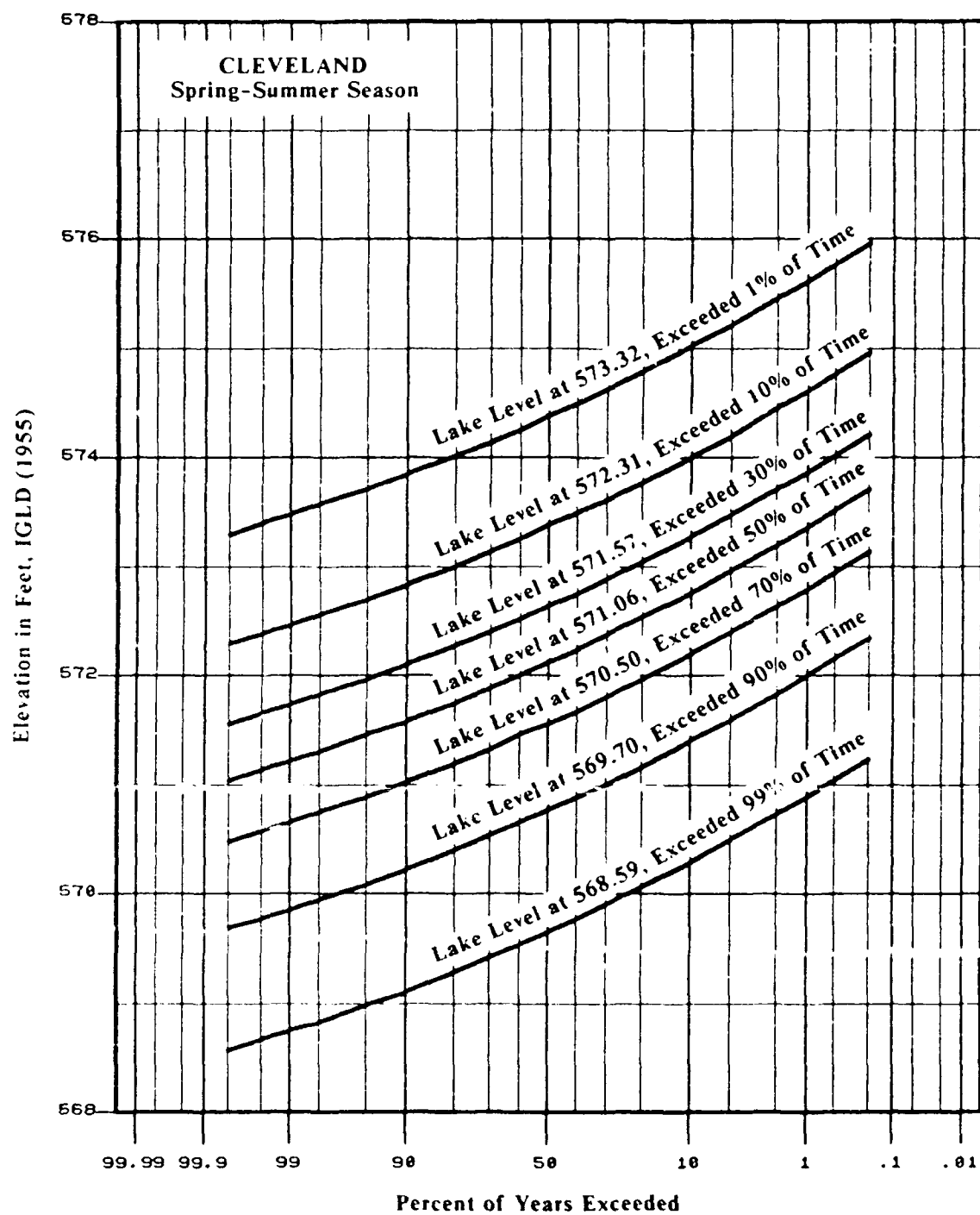


Figure 6. Conditional frequency curves, Cleveland, spring-summer season.

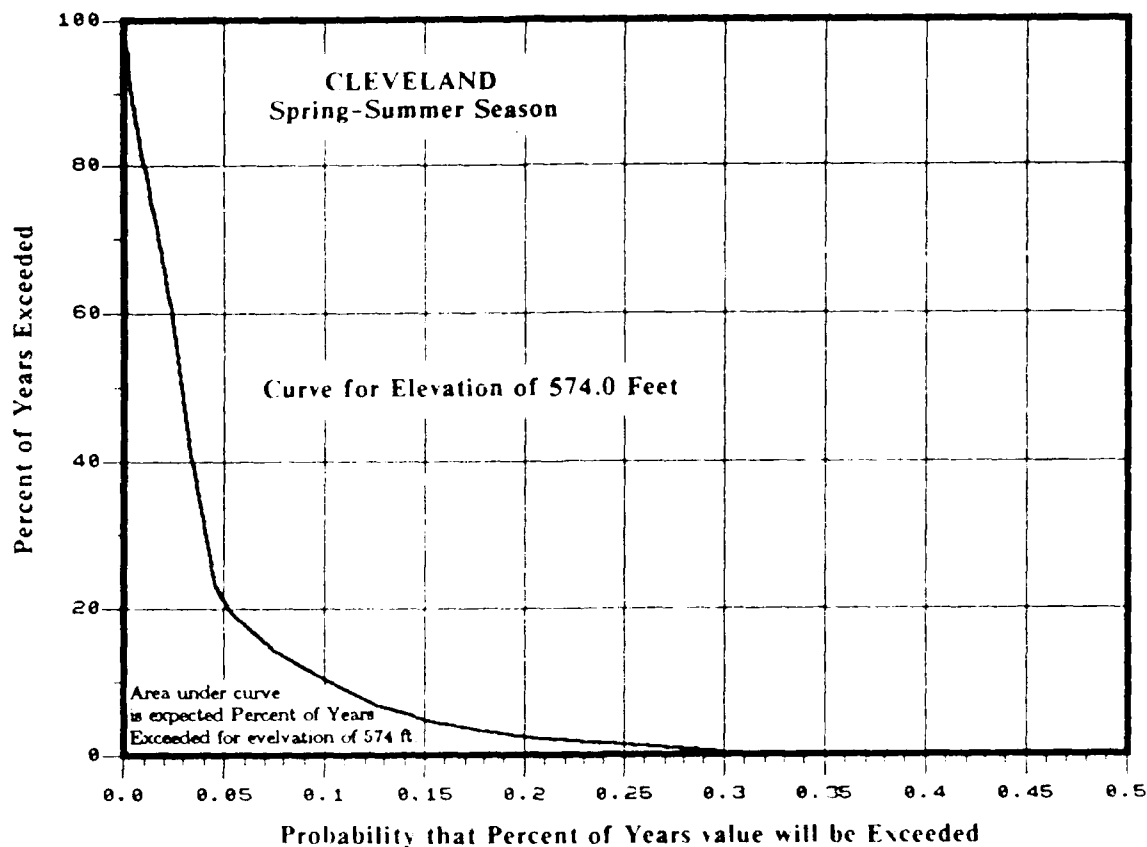


Figure 7. Graphical representation of applying the total probability equation.

## Results

The final results were found by combining the computed "frequency curves" for each of the seasons. This is done by the union of probabilities. This equation is:

$$P_c = 100[1 - (1 - P_1/100)(1 - P_2/100)]$$

where:  $P_c$  = the combined frequency value in percent for the selected elevation,

$P_1$  = the frequency value in percent for season 1 for selected elevation, and

$P_2$  = the frequency value in percent for season 2 for selected elevation.

Lake elevation expectation curves were computed for Buffalo harbor and Cleveland by the procedure described herein. The monthly duration curves were based on the period 1860-1988 while the wind setup curves were based generally on the 1900-1988 period. Therefore, these curves should be fairly representative of the 1860-1988 period. The observed instantaneous annual maximums have been assigned plotting positions and plotted along with the derived curves on Figures 8 and 9. The "1% of

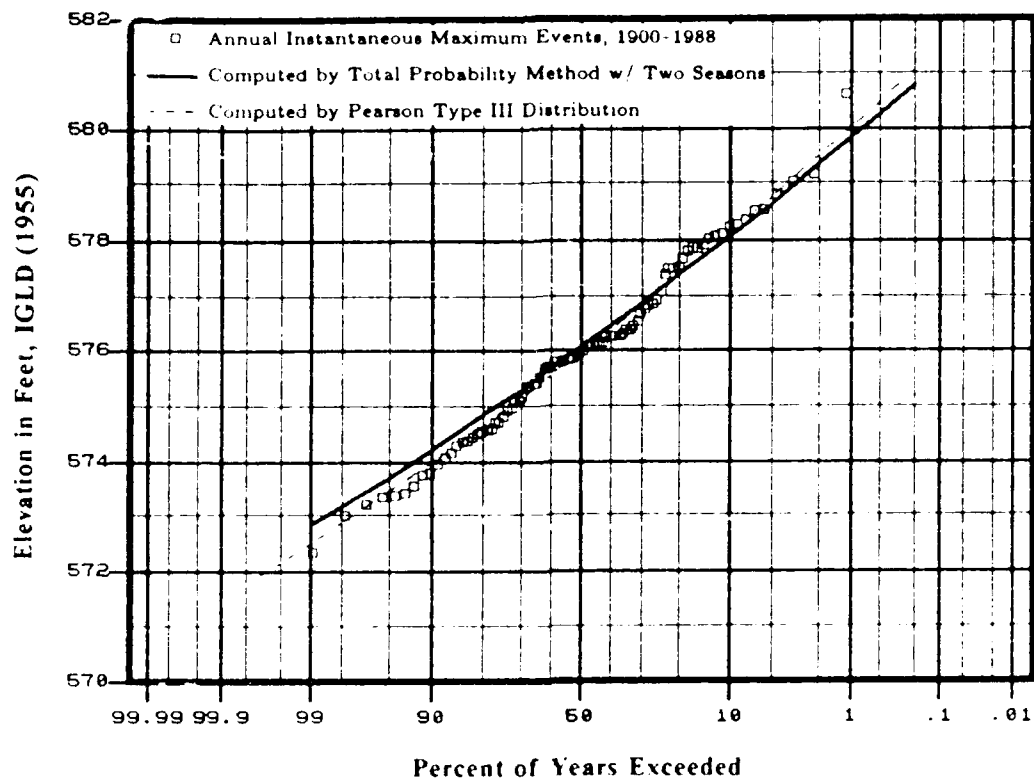


Figure 8. Frequency of annual maximums, Buffalo harbor.

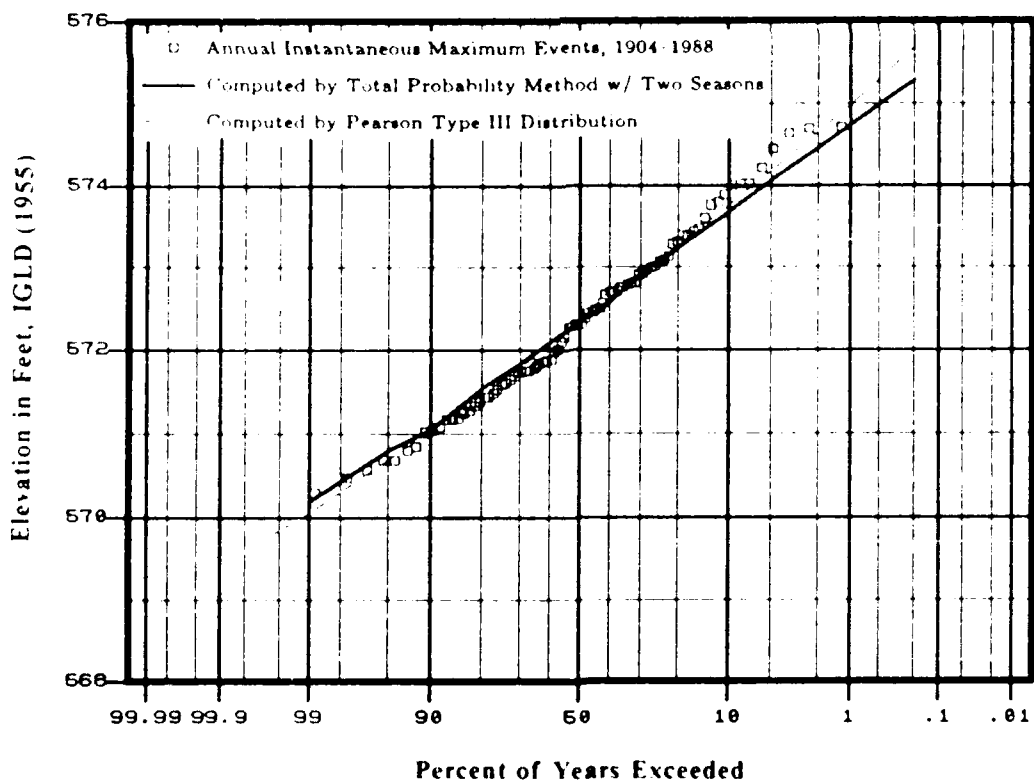


Figure 9. Frequency of annual maximums, Cleveland.

Years Exceeded" elevations computed by this procedure were 579.79 (176.72 meters) and 574.72 feet (175.17 meters) (IGLD 1955) for Buffalo harbor and Cleveland, respectively.

The utility of this procedure is in the application to gauges that have fairly short records. Mean monthly elevation duration relations based on a fairly long period are available for each of the Great Lakes. The wind setup frequency relation for an individual station may be used, or the relation could be adjusted by the "two-station comparison" procedures (Interagency Committee 1982) recommended for flood flow frequency computations. Application of these procedures to a station with a fairly short record should provide elevation expectation curves that are representative of a much longer period than the period of recorded maximum or minimum instantaneous lake elevations.

## Acknowledgments

This work was supported in part by the U.S. Army Corps of Engineers, Detroit District and the research program of the Hydrologic Engineering Center. This work was done under the supervision of Arlen Feldman, Chief Research Division and Darryl Davis, Director. I wish to thank the Great Lakes Environmental Laboratory for inviting me to participate in this exercise in water level analysis.

## References

- Benjamin, J. R. and C. A. Cornell. 1970. *Probability, Statistics, and Decision for Civil Engineers*. New York, NY: McGraw-Hill Book Co.
- Interagency Advisory Committee on Water Data. 1982. *Guidelines for Determining Flood Flow Frequency*, Bulletin 17B. Reston, VA: Hydrology Subcommittee.
- U.S. Army Corps of Engineers. 1989. *CFA, Coincident Frequency Analysis*. Computer Program. Davis, CA: Hydrologic Engineering Center.

## TECHNICAL PAPER SERIES

(\$2 per paper)

TP-1	Use of Interrelated Records to Simulate Streamflow	TP-37	Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes
TP-2	Optimization Techniques for Hydrologic Engineering	TP-38	Water Quality Evaluation of Aquatic Systems
TP-3	Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs	TP-39	A Method for Analyzing Effects of Dam Failures in Design Studies
TP-4	Functional Evaluation of a Water Resources System	TP-40	Storm Drainage and Urban Region Flood Control Planning
TP-5	Streamflow Synthesis for Ungaged Rivers	TP-41	HEC-5C, A Simulation Model for System Formulation and Evaluation
TP-6	Simulation of Daily Streamflow	TP-42	Optimal Sizing of Urban Flood Control Systems
TP-7	Pilot Study for Storage Requirements for Low Flow Augmentation	TP-43	Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
TP-8	Worth of Streamflow Data for Project Design - A Pilot Study	TP-44	Sizing Flood Control Reservoir Systems by Systems Analysis
TP-9	Economic Evaluation of Reservoir System Accomplishments	TP-45	Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
TP-10	Hydrologic Simulation in Water-Yield Analysis	TP-46	Spatial Data Analysis of Nonstructural Measures
TP-11	Survey of Programs for Water Surface Profiles	TP-47	Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
TP-12	Hypothetical Flood Computation for a Stream System	TP-48	Direct Runoff Hydrograph Parameters Versus Urbanization
TP-13	Maximum Utilization of Scarce Data in Hydrologic Design	TP-49	Experience of HEC in Disseminating Information on Hydrological Models
TP-14	Techniques for Evaluating Long-Term Reservoir Yields	TP-50	Effects of Dam Removal: An Approach to Sedimentation
TP-15	Hydrostatistics - Principles of Application	TP-51	Design of Flood Control Improvements by Systems Analysis: A Case Study
TP-16	A Hydrologic Water Resource System Modeling Techniques	TP-52	Potential Use of Digital Computer Ground Water Models
TP-17	Hydrologic Engineering Techniques for Regional Water Resources Planning	TP-53	Development of Generalized Free Surface Flow Models Using Finite Element Techniques
TP-18	Estimating Monthly Streamflows Within a Region	TP-54	Adjustment of Peak Discharge Rates for Urbanization
TP-19	Suspended Sediment Discharge in Streams	TP-55	The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
TP-20	Computer Determination of Flow Through Bridges	TP-56	Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
TP-21	An Approach to Reservoir Temperature Analysis	TP-57	Flood Damage Assessments Using Spatial Data Management Techniques
TP-22	A Finite Difference Method for Analyzing Liquid Flow in Variably Saturated Porous Media	TP-58	A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
TP-23	Uses of Simulation in River Basin Planning	TP-59	Testing of Several Runoff Models on an Urban Watershed
TP-24	Hydroelectric Power Analysis in Reservoir Systems	TP-60	Operational Simulation of a Reservoir System with Pumped Storage
TP-25	Status of Water Resource Systems Analysis	TP-61	Technical Factors in Small Hydropower Planning
TP-26	System Relationships for Panama Canal Water Supply	TP-62	Flood Hydrograph and Peak Flow Frequency Analysis
TP-27	System Analysis of the Panama Canal Water Supply	TP-63	HEC Contribution to Reservoir System Operation
TP-28	Digital Simulation of an Existing Water Resources System	TP-64	Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
TP-29	Computer Applications in Continuing Education	TP-65	Feasibility Analysis in Small Hydropower Planning
TP-30	Drought Severity and Water Supply Dependability	TP-66	Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
TP-31	Development of System Operation Rules for an Existing System by Simulation	TP-67	Hydrologic Land Use Classification Using LANDSAT
TP-32	Alternative Approaches to Water Resource System Simulation	TP-68	Interactive Nonstructural Flood-Control Planning
TP-33	System Simulation for Integrated Use of Hydroelectric and Thermal Power Generation	TP-69	Critical Water Surface by Minimum Specific Energy Using the Parabolic Method
TP-34	Optimizing Flood Control Allocation for a Multipurpose Reservoir	TP-70	Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model
TP-35	Computer Models for Rainfall-Runoff and River Hydraulic Analysis		
TP-36	Evaluation of Drought Effects at Lake Atitlan		

- TP-71 Determination of Land Use from Satellite Imagery for Input to Hydrologic Models
- TP-72 Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality
- TP-73 Flood Mitigation Planning Using HEC-SAM
- TP-74 Hydrographs by Single Linear Reservoir Model
- TP-75 HEC Activities in Reservoir Analysis
- TP-76 Institutional Support of Water Resource Models
- TP-77 Investigation of Soil Conservation Service Urban Hydrology Techniques
- TP-78 Potential for Increasing the Output of Existing Hydroelectric Plants
- TP-79 Potential Energy and Capacity Gains from Flood Control Storage Reallocation at Existing U. S. Hydropower Reservoirs
- TP-80 Use of Non-Sequential Techniques in the Analysis of Power Potential at Storage Projects
- TP-81 Data Management Systems for Water Resources Planning
- TP-82 The New HEC-1 Flood Hydrograph Package
- TP-83 River and Reservoir Systems Water Quality Modeling Capability
- TP-84 Generalized Real-Time Flood Control System Model
- TP-85 Operation Policy Analysis: Sam Rayburn Reservoir
- TP-86 Training the Practitioner: The Hydrologic Engineering Center Program
- TP-87 Documentation Needs for Water Resources Models
- TP-88 Reservoir System Regulation for Water Quality Control
- TP-89 A Software System to Aid in Making Real-Time Water Control Decisions
- TP-90 Calibration, Verification and Application of a Two-Dimensional Flow Model
- TP-91 HEC Software Development and Support
- TP-92 Hydrologic Engineering Center Planning Models
- TP-93 Flood Routing Through a Flat, Complex Flood Plain Using a One-Dimensional Unsteady Flow Computer Program
- TP-94 Dredged-Material Disposal Management Model
- TP-95 Infiltration and Soil Moisture Redistribution in HEC-1
- TP-96 The Hydrologic Engineering Center Experience in Nonstructural Planning
- TP-97 Prediction of the Effects of a Flood Control Project on a Meandering Stream
- TP-98 Evolution in Computer Programs Causes Evolution in Training Needs: The Hydrologic Engineering Center Experience
- TP-99 Reservoir System Analysis for Water Quality
- TP-100 Probable Maximum Flood Estimation - Eastern United States
- TP-101 Use of Computer Program HEC-5 for Water Supply Analysis
- TP-102 Role of Calibration in the Application of HEC-6
- TP-103 Engineering and Economic Considerations in Formulating
- TP-104 Modeling Water Resources Systems for Water Quality
- TP-105 Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
- TP-106 Flood-Runoff Forecasting with HEC-1F
- TP-107 Dredged-Material Disposal System Capacity Expansion
- TP-108 Role of Small Computers in Two-Dimensional Flow Modeling
- TP-109 One-Dimensional Model For Mud Flows
- TP-110 Subdivision Froude Number
- TP-111 HEC-5Q: System Water Quality Modeling
- TP-112 New Developments in HEC Programs for Flood Control
- TP-113 Modeling and Managing Water Resource Systems for Water Quality
- TP-114 Accuracy of Computed Water Surface Profiles - Executive Summary
- TP-115 Application of Spatial-Data Management Techniques in Corps Planning
- TP-116 The HEC's Activities in Watershed Modeling
- TP-117 HEC-1 and HEC-2 Applications on the MicroComputer
- TP-118 Real-Time Snow Simulation Model for the Monongahela River Basin
- TP-119 Multi-Purpose, Multi-Reservoir Simulation on a PC
- TP-120 Technology Transfer of Corps' Hydrologic Models
- TP-121 Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
- TP-122 The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
- TP-123 Developing and Managing a Comprehensive Reservoir Analysis Model
- TP-124 Review of the U.S. Army Corps of Engineering Involvement With Alluvial Fan Flooding Problems
- TP-125 An Integrated Software Package for Flood Damage Analysis
- TP-126 The Value and Depreciation of Existing Facilities: The Case of Reservoirs
- TP-127 Floodplain-Management Plan Enumeration
- TP-128 Two-Dimensional Floodplain Modeling
- TP-129 Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
- TP-130 Estimating Sediment Delivery and Yield on Alluvial Fans
- TP-131 Hydrologic Aspects of Flood Warning - Preparedness Programs
- TP-132 Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
- TP-133 Predicting Deposition Patterns in Small Basins
- TP-134 Annual Extreme Lake Elevations by Total Probability Theorem

REPORT DOCUMENTATION PAGE				Form Approved OMB No. 0704-0188	
1a. REPORT SECURITY CLASSIFICATION UNCLASSIFIED			1b. RESTRICTIVE MARKINGS		
2a. SECURITY CLASSIFICATION AUTHORITY			3. DISTRIBUTION / AVAILABILITY OF REPORT		
2b. DECLASSIFICATION / DOWNGRADING SCHEDULE					
4. PERFORMING ORGANIZATION REPORT NUMBER(S) Technical Paper No. 134			5. MONITORING ORGANIZATION REPORT NUMBER(S)		
6a. NAME OF PERFORMING ORGANIZATION Hydrologic Engineering Center		6b. OFFICE SYMBOL (If applicable) CEWRC-HEC	7a. NAME OF MONITORING ORGANIZATION U.S. Army Corps of Engineers Water Resources Support Center		
6c. ADDRESS (City, State, and ZIP Code) 609 Second Street Davis, California 95616		7b. ADDRESS (City, State, and ZIP Code) Casey Building #2594 Fort Belvoir, Virginia 22060			
8a. NAME OF FUNDING / SPONSORING ORGANIZATION		8b. OFFICE SYMBOL (If applicable)	9. PROCUREMENT INSTRUMENT IDENTIFICATION NUMBER		
8c. ADDRESS (City, State, and ZIP Code)		10. SOURCE OF FUNDING NUMBERS			
		PROGRAM ELEMENT NO.	PROJECT NO.	TASK NO.	WORK UNIT ACCESSION NO.
11. TITLE (Include Security Classification) Annual Extreme Lake Elevations by Total Probability Theorem					
12. PERSONAL AUTHOR(S) Harold E. Kubik, P.E.					
13a. TYPE OF REPORT Technical Paper		13b. TIME COVERED FROM _____ TO _____		14. DATE OF REPORT (Year, Month, Day)	
				15. PAGE COUNT 10	
16. SUPPLEMENTARY NOTATION Presented at the Great Lakes Water Level Forecasting and Statistics Symposium, May 17 & 18, 1990, Windsor, Ontario, Canada					
17. COSATI CODES			18. SUBJECT TERMS (Continue on reverse if necessary and identify by block number)		
FIELD	GROUP	SUB-GROUP	Lake elevation, storm surge, total probability theorem, statistics, coincident frequency, Lake Erie		
19. ABSTRACT (Continue on reverse if necessary and identify by block number)  Annual extreme water levels on the Great Lakes, whether maximums or minimums, have a high serial dependence. Therefore, application of traditional frequency analysis techniques must be interpreted in a different manner and more sophisticated statistical techniques must be applied to account for this dependence. Decomposition of the annual extremes into two parts, one containing the highly dependent part and the other containing the random part, is one method of dealing with the dependence in the lake elevations. Appropriate statistical analyses can be applied to the separate parts and then the individual results combined to obtain the final frequency relation. This study develops mean monthly lake elevation duration curves to represent the dependent part and wind setup frequency curves for the random part. These parts are then combined by application of the total probability theorem.					
20. DISTRIBUTION / AVAILABILITY OF ABSTRACT <input checked="" type="checkbox"/> UNCLASSIFIED/UNLIMITED <input type="checkbox"/> SAME AS RPT. <input type="checkbox"/> DTIC USERS			21. ABSTRACT SECURITY CLASSIFICATION		
22a. NAME OF RESPONSIBLE INDIVIDUAL Darryl W. Davis, Director, HEC			22b. TELEPHONE (Include Area Code) (916) 756-1104		22c. OFFICE SYMBOL CEWRC-HEC